DEPARTMENT OF LABOR AND INDUSTRY

CODE CHANGE PROPOSAL FORM

(Must be submitted electronically)

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Date: 4-19-18

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Model Code: 2018 International Residential Code (IRC)

Telephone number: 612-720-4639

Code or Rule Section: MR 1309.0404 Section R404.1

Firm/Association affiliation, if any: Oswell Engineering/BAM

Code or rule section to be changed: 2018 IRC, Section R404.1 (Concrete and Masonry Cantilevered Foundation Walls)

Intended for Technical Advisory Group; Residential Building Code TAG

General Information	<u>Yes</u>	<u>No</u>
A. Is the proposed change unique to the State of Minnesota?	\boxtimes	
B. Is the proposed change required due to climatic conditions of Minnesota?		\boxtimes
C. Will the proposed change encourage more uniform enforcement?	\boxtimes	
D. Will the proposed change remedy a problem?		\boxtimes
E. Does the proposal delete a current Minnesota Rule, chapter amendment?		\boxtimes
F. Would this proposed change be appropriate through the ICC code		
development process?		\boxtimes

Proposed Language

1. The proposed code change is meant to:

change language contained the model code book? If so, list section(s).

 \boxtimes change language contained in an existing amendment in Minnesota Rule? If so, list Rule part(s). MN 1309.404 section R404 Tables R404.1.1(5) (6) and (7)

delete language contained in the model code book? If so, list section(s).

delete language contained in an existing amendment in Minnesota Rule? If so, list Rule part(s).

add new language that is not found in the model code book or in Minnesota Rule.

- 2. Is this proposed code change required by Minnesota Statute? If so, please provide the citation. No
- 3. Provide *specific* language you would like to see changed. Indicate proposed new words with <u>underlining</u> and words proposed to be deleted. Include the entire code (sub) section or rule subpart that contains your proposed changes.

Remove Tables R404.1.1 (5), (6), and (7) from the MN rules and replace them with the following paragraph added/rewrite of the R404.1 exception:

Exception 1: Cantilevered concrete and masonry foundation walls that do not have permanent lateral support at the top shall be constructed as follows when the unbalanced grade height supported by the foundation wall measured from the top of the slab on grade is no more than 30".

- 1. The foundation wall shall be at least 8" thick.
- 2. Final grade shall be at least 6" below the top of the foundation wall. The backfill soil shall be of soil group I, II, or III per Table R405.1.
- 3. The maximum frame wall height atop the foundation wall shall be 7'.
- 4. The foundation wall shall be supported in tight contact by a concrete slab-on-grade at least 3.5" in thickness.
- 5. The concrete strip footing supporting the foundation wall shall be at least 20" wide by 8" thick centered on the foundation wall but not less than the size noted in Table R403. The footing elevation may be lowered below the slab-on-grade as required for frost protection.
- 6. The minimum load-bearing pressure of the soil below the footing shall be 1500 pounds per square foot.
- 7. The foundation wall and strip footing shall be connected together with #4 grade 40 minimum by 36" minimum long rebar dowels centered in the foundation wall at a maximum of 24" on center with at least 5" embedment into the strip footing. For masonry walls the cores containing dowels shall be grouted solid full height. Alternative dowel bar sizes and spacings having an equivalent cross-sectional area of reinforcement per lineal foot of wall shall be permitted provided the spacing of the dowels does not exceed 72" on center.
- 8. Anchor bolts shall be installed in accordance with section R403.1.6.
- 9. Horizontal reinforcing for concrete walls shall be installed in accordance with Table R404.1.2(1).
- 10. Mortar shall be type M or S and masonry shall be laid in running bond.
- 11. Grout and concrete shall have a 28 day compressive strength of at least 3000 psi.

Exception 2: Cantilevered concrete and masonry foundation walls that do not have permanent lateral support at the top shall be constructed as follows when the unbalanced grade height supported by the foundation wall measured from the top of the slab on grade more than 30" but no more than 48". Cantilevered foundation walls supporting more than 48" of unbalanced grade shall be designed in accordance with accepted engineering practice.

- 1. The foundation wall shall be at least 12" thick.
- 2. Final grade shall be at least 6" below the top of the foundation wall. The backfill soil shall be of soil group I, II, or III per Table R405.1.
- 3. The maximum frame wall height atop the foundation wall shall be 5'-6".
- 4. The foundation wall shall be supported in tight contact by a concrete slab-on-grade at least 3.5" in thickness.
- 5. The concrete strip footing supporting the foundation wall shall be at least 28" wide by 8" thick centered on the foundation wall but not less than the size noted in Table R403. The footing elevation may be lowered below the slab-on-grade as required for frost protection.
- 6. The minimum load-bearing pressure of the soil below the footing shall be 1500 pounds per square foot.

- 7. The foundation wall and strip footing shall be connected together with #4 grade 40 minimum by 36" minimum long rebar dowels centered in the foundation wall at a maximum of 16" on center with at least 5" embedment into the strip footing. For masonry walls the cores containing dowels shall be grouted solid full height. Alternative dowel bar sizes and spacings having an equivalent cross-sectional area of reinforcement per lineal foot of wall shall be permitted provided the spacing of the dowels does not exceed 72" on center.
- 8. Anchor bolts shall be installed in accordance with section R403.1.6.
- 9. Horizontal reinforcing for concrete walls shall be installed in accordance with Table R404.1.2(1).
- 10. Mortar shall be type M or S and masonry shall be laid in running bond.
- 11. Grout and concrete shall have a 28 day compressive strength of at least 3000 psi.
- Will this proposed code change impact other sections of a model code book or an amendment in Minnesota Rule? If so, please list the affected sections or rule parts. No

Need and Reason

- 1. Why is the proposed code change needed? The current cantilevered tables are confusing, not being used appropriately, and are based on questionable design (factor of safety of 1 for overturning, no use of wind, questionable dowel and side spanning use)
- Why is the proposed code change a reasonable solution? It clarifies confusing language and replaces it with something that is already industry standard and suitable for all frost depth locations in the state.
- 3. What other considerations should the TAG consider? None

Cost/Benefit Analysis

- Will the proposed code change increase or decrease costs? Please explain. Design will be required for higher wall cases that were uncommon but were included in the MN rules current tables.
- If there is an increased cost, will this cost be offset by a safety or other benefit? Please explain. The new language actually is structurally adequate and simpler to install and inspection since it is more uniform
- 3. Are there any enforcement or compliance cost increases or decreases with the proposed code change? Please explain.

No as it is already in effect in the current MN rules.

4. Will the cost of complying with the proposed code change in the first year after the rule takes effect exceed \$25,000 for any one small business or small city? A small business is any business that has less than 50 full-time employees. A small city is any statutory or home rule charter city that has less than ten full-time employees. Please explain.

Potentially, but it would only be in the design cost as the final construction would be about the same as it was before

Regulatory Analysis

- 1. What parties or segments of industry are affected by this proposed code change? Building officials, inspectors, builders, excavators, designers, and foundation contractors
- 2. What are the probable costs to the agency and to any other State agencies of implementing and enforcing of the proposed rule? Is there an anticipated effect on state revenues? None

- 3. Are there less costly intrusive methods for achieving the purpose of the proposed rule? No
- Can you think of other means or methods to achieve the purpose of the proposed code change? If so, please explain what they are and why your proposed change is the preferred method or means to achieve the desired result.
- 5. What are the probable costs of complying with the proposed rule, including the portion of the total costs that will be borne by identifiable categories of affected parties, such as separate classes of governmental units, businesses, or individuals? None
- 6. What are the probable costs or consequences of not adopting the proposed rule, including those costs or consequences borne by identifiable categories of affected parties, such as separate classes of government units, businesses, or individuals? Potential improper installation of cantilevered walls with associated distresses and failures
- Are you aware of any federal regulation or federal requirement related to this proposed code change? If so, please list the federal regulation or requirement and your assessment of any differences between the proposed rule and the federal regulation or requirement. No
- Please include an assessment of the cumulative effect of the rule with other federal and state regulations related to the specific purpose of the rule. None

***Note: Incomplete forms may be returned to the submitter with instruction to complete the form. Only completed forms can considered by the TAG.

TABLE R404.1.1(5)

CANTILEVERED CONCRETE AND MASONRY FOUNDATION WALLS^{e,f,g,h,k,l,m,n}

Max. Unbalanced Backfill Height (inches) ^{a,b,j}	Max. Frame Wall Height (feet) ⁱ	Fnd. Wall Thickness (inches)	Soil Class	Concrete Footing Width x Depth (inches) ^{c,g}	#4 Footing Dowel o.c. Spacing (inches) ^{d,k}	#4 Footing Hook o.c. Spacing (inches) ^{d,k}	#5 Footing Dowel o.c. Spacing (inches) ^{d,k}	#5 Footing Hook o.c. Spacing (inches) ^{d,k}
≤ 30	7'-0"	8	GW, GP, SW, SP	18 x 8	32	64	40	72
	7'-0''	8	GM, GC, SM, SM-SC, ML	18 x 8	24	54	32	72
	7'-0"	8	SC, ML-CL, I-CL	18 x 8	24	48	32	72
	7'-0"	10	GW, GP, SW, SP	18 x 8	56	72	64	72
	7'-0"	10	GM, GC, SM, SM-SC, ML	18 x 8	56	72	64	72
	7'-0"	10	SC, ML-CL, I-CL	18 x 8	56	72	64	72
	7'-0"	12	GW, GP, SW, SP	18 x 8	64	72	72	72
	7'-0"	12	GM, GC, SM, SM-SC, ML	18 x 8	64	72	72	72
	7'-0"	12	SC, ML-CL, I-CL	18 x 8	64	72	72	72
$>30 \text{ to} \le 48$	5'-6"	8	GW, GP, SW, SP	24 x 8	16	32	16	48

5'-6'	,,	10	GW, GP, SW, SP	24 x 8	16	40	24	64
5'-6'	"	10	GM, GC, SM, SM-SC, ML	26 x 8	16	32	16	48
5'-6'	"	12	GW, GP, SW, SP	24 x 8	24	48	32	72
5'-6'	"	12	GM, GC, SM, SM-SC, ML	26 x 8	16	32	24	56
5'-6'	,,	12	SC, ML-CL, I-CL	28 x 8	16	32	16	48

a. Final grade shall be at least 6 inches below the top of the foundation wall.

b. A 3-1/2 inches minimum thick concrete slab-on-grade is to be poured tight against the bottom of the foundation wall.

c. The concrete strip footing supporting the foundation shall be centered on the foundation wall and sized in accordance with Table R404.1.1(5), but not less than the size noted in Table R403.1(1), Table R403.1(2), or Table R403.1(2). The footing elevation may be lowered below the slab-on-grade as required for frost protection.

- d. Concrete Foundation wall and strip footing shall be connected together with Grade 60 minimum by 24 inches minimum long rebar dowel or hook centered in the foundation wall spaced in accordance with Table R404.1.1(5) with at least 5 inches embedment into the strip footing. Hooks are to have a 6 inches minimum long horizontal portion in the footing. For masonry walls the cores containing dowels or hooks shall be grouted solid full height. The dowel and hook vertical portion is to extend at least 14 inches above the top of the floor slab.
- e. Anchor bolts shall be installed in accordance with section R403.1.6.
- f. Mortar shall be Type M or S and masonry shall be laid in running bond.
- g. Grout and concrete for walls shall have a minimum 28 day compressive strength of at least 3,000 psi.
- h. Cantilevered foundation walls supporting more than 48 inches of unbalanced grade shall be designed in accordance with accepted engineering practice.
- i. Maximum frame wall height from top of foundation wall.
- j. Maximum unbalanced fill height from top of concrete slab to grade.
- k. Concrete for footings shall have a minimum 28 day compressive strength of at least 5,000 psi per Table R402.2.
- 1. Horizontal reinforcing is required in the walls per R404.1.2(1).
- m. The minimum allowable soil bearing capacity is to be at least 1500 psf.
- n. The wind load is to be 115 mph exposure C or less.

Subp. 6. IRC Table R404.1.1(5). Section R404 is amended by adding a new table as follows:

TABLE R404.1.1(5)

CANTILEVERED CONCRETE AND MASONRY FOUNDATION WALLS^{6,fg,h,k,l,m,n}

d,k d,k	#				#			ſ	# ~	4
#5 Footi Hook o. Spacing (inches)	72	72	72	72	72	72	72	72	72	48
#5 Footing Dowel o.c. Spacing (inches) ^{d,k}	40	32	32	64	64	64	72	72	72	16
#4 Footing Hook o.c. Spacing (inches) _{d,k}	64	54	48	72	72	72	72	72	72	32
#4 Footing Dowel o.c. Spacing (inches) ^{d,k}	32	24	24	56	56	56	64	64	64	16
Concrete Footing Width x Depth (inches) ^{c,g}	18 x 8	18 x 8	18 x 8	18 x 8	18 x 8	18 x 8	18 x 8	18 x 8	18 x 8	24 x 8
Soil Class	GW, GP, SW, SP	GM, GC, SM, SM-SC, ML	SC, ML-CL, I-CL	GW, GP, SW, SP	GM, GC, SM, SM-SC, ML	SC, ML-CL, I-CL	GW, GP, SW SP	GM, GC, SM, SM-SC, ML	SC, ML-CL, I-CL	GW, GP, SW, SP
Fnd. Wall Thickness (inches)	œ	ø	∞	10	10	10	12	12	12	∞
Max. Frame Wall Height	(reet) 7'-0''	7,-0,,	7,-0,,	7,-0,,	7,-0,,	7°-0"	7,-0,,	7,-0"	7,-0,,	5,-6"
Max. Unbalanced Backfill Height	<pre>(incnes) ~ '</pre>									>30 to ≤48

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	5°-6"	10	GW, GP, SW, SP	24 x 8	16	40	24	64	
	5°-6"	10	GM, GC, SM, SM-SC, ML	26 x 8	16	32	16	48	#20
	5'-6''	12	GW, GP, SW, SP	24 x 8	24	48	32	72	
	5°-6"	12	GM, GC, SM, SM-SC, ML	26 x 8	16	32	24	56	
	5,-6"	12	SC, ML-CL, I-CL	28 x 8	16	32	16	48	40
с. р. з	Final grade shall be at Final grade shall be at TA 3-1/2 inches minimu The concrete strip foot than the size noted in T	least 6 inches below th im thick concrete slab- ing supporting the four fable R403 1(1) Table	te top of the foundatio on-grade is to be pour ndation shall be center BA02 1(2) or T-A12	n wall. red tight against red on the found	the bottom of th dation wall and s	ie foundation wall.	e with Table R404	.1.1(5), but n	ot less
d.	frost protection. Concrete Foundation w the foundation wall spa	vall and strip footing sl aced in accordance wit	hall be connected toge h Table R404.1.1(5) v	ther with Grade vith at least 5 in	60 minimum by ches embedment	/ 24 inches minim	um long rebar dow ing. Hooks are to	-graue as req vel or hook ce have a 6 incl	urred for intered in
e.	minimum long horizon hook vertical portion is Anchor bolts shall be ii	ntal portion in the footi. s to extend at least 14 i nstalled in accordance	ng. For masonry wall nches above the top o with section R403.1.6	ls the cores cont f the floor slab.	aining dowels or	hooks shall be gr	outed solid full he	ight. The do	vel and
f.	Mortar shall be Type N	A or S and masonry shi	all be laid in running t	ond.					
ы. н. ю	Grout and concrete for Cantilevered foundation Maximum frame wall h	walls shall have a min in walls supporting mo neight from top of foun	imum 28 day compre. re than 48 inches of u idation wall.	ssive strength o nbalanced grade	f at least 3,000 p e shall be designe	si. ed in accordance w	vith accepted engir	teering practi	ce.
	Maximum unbalanced	fill height from top of	concrete slab to grade						
k.	Concrete for footings sl	hall have a minimum 2	38 day compressive str	rength of at leas	t 5,000 psi per T	able R402.2.			
I. 1	Horizontal reinforcing i	is required in the walls	per R404.1.2(1).						
m.	The minimum allowabl	le soil bearing capacity	is to be at least 1500	psf.					
'n.	The wind load is to be 1	115 mph exposure C or	r less.						

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CASE #1								
Nominal Wall Thickness =	8.0	in		Wall	Type =	Masonr	у	
Wall Height (above TOF) =	3.33	ft		Soil	Type =	Sand		
Footing Width =	1.50	ft	Allowa	ble Bea	aring =	1500	psf	
Wall Reveal =	6	in	Activ	ve Soil	Load =	30	psf/ft	
Unbalance Height =	2.50	ft		Soil W	eight =	125	pcf	
Footing Thickness =	8	in		Wind	Load =	17.3	psf	
Front Wall Offset =	0.42	ft	115 mph expo	osure C	@ 15'	(area 4 i	nward)	
Back Wall Offset =	0.42	ft						
Basement Clear Height =	10.00	ft						
Soil P =	94	plf	@ x = 0.83	ft	M =	78	lb-ft	
Wind P =	69	plf	@ x = 3.00	ft	M =	208	lb-ft	
			Total Mo a	t top of	slab =	286	lb-ft	
Overturning Resistance:								
Wall Weight =	178	plf	@ y = 0.75	ft	M =	133	lb-ft	
Footing Weight =	150	plf	@ y = 0.75	ft	M =	113	lb-ft	
Soil Weight =	147	plf	@ y = 1.29	ft	M =	190	lb-ft	
DL from Above =	200	plf	@ y = 0.83	ft	M =	167	lb-ft	
				Tot	al Mr =	603	lb-ft	
Bearing:					FS =	2.11	OK > 1	1.5
e =	0.42	ft						
B/6 =	0.25	ft		Foot	ing Co	nnection	:	
Bearing Pressure =	1378	psf			Mu =	5.5	k-in	
Ratio =	0.92		OK @ 4/3	Loc	ation =	C 3 75	(C or (C)
Rear Footing Bending:					Ru =	0.036	nn ksi	
Mu =	0 59	k-in	R	ehar G	rade =	60	ksi	
S =	128	in3			F'c =	5	ksi	
fh =	5	nsi			. c R1 =	0.8	Nor	
Fh =	318	nsi	OK			15 00		
Sr =	2	in3	ÖN		rho =	0 0006		
Rea'd Thickness =	10	in	ок	rho	min =	0.0008		
	1.0		ÖN		Δs =	0.0000	in2/ft	
Wall Check				Bar	Size =	4	5	
Mu $(18" above TOS) =$	1 31	k-in		Bar	Δrea =	0.20	0.31	in2
$\mathbf{A} =$	36	in2	1	Minimu	m l d =	10.2	12 7	in
S =	90	in3		Actu	alld=	5.0	5.0	in
fa =	8	nsi	Dov	vel Sna	ai La acina =	32	40	in
fb =	14	nsi	Ho	ok Sna	acing =	65	101	in
Fb =	500	psi	ОК	on ope				
ft =	7	nsi						
Ft =	25	psi	ОК					
		P						

CASE #2								
Nominal Wall Thickness =	10.0	in		Wall	Type =	Masonr	у	
Wall Height (above TOF) =	3.33	ft		Soil	Type =	SC		
Footing Width =	1.50	ft	Allowa	ble Be	aring =	1500	psf	
Wall Reveal =	6	in	Activ	/e Soil	Load =	45	psf/ft	
Unbalance Height =	2.50	ft		Soil W	eight =	125	pcf	
Footing Thickness =	8	in		Wind	Load =	17.3	psf	
Front Wall Offset =	0.33	ft	115 mph expo	osure C	0 @ 15'	(area 4 i	nward))
Back Wall Offset =	0.33	ft						
Basement Clear Height =	10.00	ft						
Soil P =	141	plf	@ x = 0.00	ft	M =	0	lb-ft	
Wind P =	69	plf	@ x = 3.00	ft	M =	208	lb-ft	
			Total Mo a	t top of	f slab =	208	lb-ft	
Overturning Resistance:								
Wall Weight =	222	plf	@ y = 0.75	ft	M =	167	lb-ft	
Footing Weight =	150	plf	@ y = 0.75	ft	M =	113	lb-ft	
Soil Weight =	118	plf	@ y = 1.33	ft	M =	157	lb-ft	
DL from Above =	200	plf	@ y = 0.92	ft	M =	183	lb-ft	
				Tot	al Mr =	620	lb-ft	
Bearing:					FS =	2.98	OK >	1.5
e =	0.30	ft						
B/6 =	0.25	ft		Foot	ing Co	nnectior	1:	
Bearing Pressure =	1024	psf			Mu =	4.0	k-in	
Ratio =	0.68		OK @ 4/3	Loc	ation =	С	(C or (0)
					d =	4.75	in	
Rear Footing Bending:					Ru =	0.016	ksi	
Mu =	0.38	k-in	R	ebar G	Frade =	60	ksi	
S =	128	in3			F'c =	5	ksi	
fb =	3	psi			B1 =	0.8		
Fb =	318	psi	OK		m =	15.00		
Sr =	1	in3			rho =	0.0003		
Req'd Thickness =	0.8	in	OK	rhe	o min =	0.0004		
					As =	0.02	in2/ft	
Wall Check:				Bar	⁻ Size =	4	5	
Mu (18" above TOS) =	1.34	k-in		Bar	Area =	0.20	0.31	in2
A =	36	in2	I	Minimu	m Ld =	10.2	12.7	in
S =	125	in3		Actu	al Ld =	5.0	5.0	in
fa =	8	psi	Dov	vel Spa	acing =	56	70	in
fb =	11	psi	Ho	ook Spa	acing =	113	177	in
Fb =	500	psi	OK					
ft =	2	psi						
Ft =	25	psi	ОК					

CASE #3								
Nominal Wall Thickness =	12.0	in		Wall	Type =	Masonr	у	
Wall Height (above TOF) =	3.33	ft		Soil	Type =	Clay		
Footing Width =	1.50	ft	Allowa	ble Be	aring =	1500	psf	
Wall Reveal =	6	in	Activ	ve Soil	Load =	60	psf/ft	
Unbalance Height =	2.50	ft		Soil W	eight =	125	pcf	
Footing Thickness =	8	in		Wind	Load =	17.3	psf	
Front Wall Offset =	0.25	ft	115 mph expo	osure C	@ 15'	(area 4 i	nward))
Back Wall Offset =	0.25	ft						
Basement Clear Height =	10.00	ft						
Soil P =	188	plf	@ x = 0.00	ft	M =	0	lb-ft	
Wind P =	69	plf	@ x = 3.00	ft	M =	208	lb-ft	
			Total Mo a	t top of	f slab =	208	lb-ft	
Overturning Resistance:								
Wall Weight =	266	plf	@ y = 0.75	ft	M =	200	lb-ft	
Footing Weight =	150	plf	@ y = 0.75	ft	M =	113	lb-ft	
Soil Weight =	88	plf	@ y = 1.38	ft	M =	122	lb-ft	
DL from Above =	200	plf	@ y = 1.00	ft	M =	200	lb-ft	
				Tot	al Mr =	634	lb-ft	
Bearing:					FS =	3.05	OK > '	1.5
e =	0.29	ft						
B/6 =	0.25	ft		Foot	ing Co	nnectior):	
Bearing Pressure =	1032	psf			Mu =	4.0	k-in	
Ratio =	0.69		OK @ 4/3	Loc	ation =	C	(Cor	0)
Rear Footing Bending:					Ru =	0.011	ksi	
Mu =	0.21	k-in	R	ebar G	Frade =	60	ksi	
S =	128	in3			F'c =	5	ksi	
fb =	2	psi			B1 =	0.8		
Fb =	318	, psi	ОК		m =	15.00		
Sr =	1	in3			rho =	0.0002		
Reg'd Thickness =	0.6	in	ОК	rhe	o min =	0.0002		
·					As =	0.02	in2/ft	
Wall Check:				Bar	- Size =	4	5	
Mu (18" above TOS) =	1.37	k-in		Bar	Area =	0.20	0.31	in2
A =	36	in2	1	Minimu	m Ld =	10.2	12.7	in
S =	160	in3		Actu	al Ld =	5.0	5.0	in
fa =	9	psi	Dov	vel Spa	acing =	67	84	in
fb =	9	psi	Ho	ook Spa	acing =	137	215	in
Fb =	500	psi	ОК	•	-			
ft =	0	psi						
Ft =	25	psi	ок					

CASE #4				
Nominal Wall Thickness =	8.0	in	Wall Type = Mason	r y
Wall Height (above TOF) =	4.83	ft	Soil Type = Sand	
Footing Width =	2.00	ft	Allowable Bearing = 1500	psf
Wall Reveal =	6	in	Active Soil Load = 30	psf/ft
Unbalance Height =	4.00	ft	Soil Weight = 125	pcf
Footing Thickness =	8	in	Wind Load = 17.3	psf
Front Wall Offset =	0.67	ft	115 mph exposure C @ 15' (area 4	inward)
Back Wall Offset =	0.67	ft		, i
Basement Clear Height =	10.00	ft		
Soil P =	240	plf	@ x = 1.33 ft M = 320	lb-ft
Wind P =	56	plf	@x = 4.50 ft M = 253	lb-ft
Overturning Resistance:			1 otal Mo at top of slab = 573	lb-ft
Wall Weight =	258	plf	@y=1.00 ft M= 258	lb-ft
Footing Weight =	200	plf	@v = 1.00 ft M = 200	lb-ft
Soil Weight =	361	plf	@ y = 1.67 ft M = 601	lb-ft
DL from Above =	185	plf	@ v = 1.08 ft M = 200	lb-ft
		P.1	Total $Mr = 1259$	lb-ft
Bearing:			FS = 2.20	OK > 1.5
e =	0.57	ft	10 2.20	
B/6 =	0.33	ft	Footing Connectio	n.
Bearing Pressure =	1560	nsf	$M_{\rm H} = 11.0$	k-in
Ratio =	1 04	por	OK @ 4/3 ocation = C	(C or O)
hado	1.04		d = 3.75	in
Rear Footing Bending:			$R_{\rm H} = 0.072$	ksi
Mu =	2 31	k-in	Rehar Grade = 60	ksi
S =	128	in3	$F'_{C} = 5$	ksi
fb =	18	nsi	B1 = 0.8	Nor
Fb =	318	nsi	M = 15.00	
Sr =	7	in3	rho = 0.0012	
Pog'd Thickness =	10	in	OK rbo min = 0.0012	
Req u Mickness –	1.5			in2/ft
Wall Check:			A5 = 0.07 Bor Size = 4	5
Mu $(18" above TOS) =$	2.06	k.in	$\begin{array}{c} \text{Dat Size - 4} \\ \text{Bar Area - 0.20} \end{array}$	0.31 in 2
$\Lambda =$	2.30	n-111 in2	Dai Area - 0.20 Minimum I d - 10.2	10.01 IIIZ
A	30	111Z		12.7 III 5.0 in
5 = fo =	90	ins nai	Actual Ld = 5.0	0.0 III
ta =	10	psi	Dowel Spacing = 16	∠u in 50 in
		psi	HOOK Spacing = 32	ou in
tb =	500			
fb = Fb =	500	psi	ОК	
tb = Fb = ft =	500 23	psi psi	ОК	

CASE #5								
Nominal Wall Thickness =	10.0	in		Wall [®]	Type =	Masonr	у	
Wall Height (above TOF) =	4.83	ft		Soil	Type =	SC		
Footing Width =	2.17	ft	Allowa	ble Bea	aring =	1500	psf	
Wall Reveal =	6	in	Activ	ve Soil I	Load =	45	psf/ft	
Unbalance Height =	4.00	ft		Soil We	eight =	125	pcf	
Footing Thickness =	8	in		Wind I	Load =	17.3	psf	
Front Wall Offset =	0.67	ft	115 mph expo	osure C	@ 15'	(area 4 i	nward)	
Back Wall Offset =	0.67	ft						
Basement Clear Height =	10.00	ft						
Soil P =	360	plf	@ x = 1.33	ft	M =	480	lb-ft	
Wind P =	56	plf	@ x = 4.50	ft	M =	253	lb-ft	
			Total Mo a	t top of	slab =	733	lb-ft	
Overturning Resistance:								
Wall Weight =	322	plf	@ y = 1.09	ft	M =	349	lb-ft	
Footing Weight =	217	plf	@ y = 1.09	ft	M =	235	lb-ft	
Soil Weight =	362	plf	@ y = 1.84	ft	M =	664	lb-ft	
DL from Above =	185	plf	@ y = 1.25	ft	M =	232	lb-ft	
				Tota	al Mr =	1480	lb-ft	
Bearing:					FS =	2.02	OK > 1	1.5
e =	0.68	ft						
B/6 =	0.36	ft		Footi	ing Co	nnection	:	
Bearing Pressure =	1766	psf			Mu =	14.1	k-in	
Ratio =	1.18		OK @ 4/3	Loca	ation =	C 4 75	(Cor(D)
Rear Footing Bending:					Ru =	0.058	nn ksi	
Mu =	2 32	k-in	R	lebar G	rade =	60	ksi	
S =	128	in3	•		F'c =	5	ksi	
fb =	18	psi			B1 =	0.8		
Fb =	318	psi	ОК		 m =	15.00		
Sr =	7	in3	•••		rho =	0.0010		
Rea'd Thickness =	1.9	in	ОК	rho	min =	0.0013		
					As =	0.07	in2/ft	
Wall Check:				Bar	Size =	4	5	
Mu (18" above TOS) =	3.43	k-in		Bar	Area =	0.20	0.31	in2
A =	36	in2		Minimu	m Ld =	10.2	12.7	in
S =	125	in3		Actu	al Ld =	5.0	5.0	in
fa =	11	psi	Dov	wel Spa	ncina =	16	20	in
fb =	27	psi	Ho	ook Spa	icina =	32	50	in
Fb =	500	psi	ОК					
ft =	17	psi						
Ft =	25	psi	ОК					
	-							

CASE #6								
Nominal Wall Thickness =	12.0	in		Wall	Type =	Masonr	у	
Wall Height (above TOF) =	4.83	ft		Soil	Type =	Clay		
Footing Width =	2.33	ft	Allowa	ble Bea	aring =	1500	psf	
Wall Reveal =	6	in	Activ	ve Soil	Load =	• 60	psf/ft	
Unbalance Height =	4.00	ft		Soil W	eight =	: 125	pcf	
Footing Thickness =	8	in		Wind	Load =	17.3	psf	
Front Wall Offset =	0.67	ft	115 mph expo	osure C	@ 15'	(area 4 i	nward))
Back Wall Offset =	0.67	ft						
Basement Clear Height =	10.00	ft						
Soil P =	480	plf	@ x = 1.33	ft	M =	640	lb-ft	
Wind P =	56	plf	@ x = 4.50	ft	M =	253	lb-ft	
			Total Mo a	t top of	[;] slab =	893	lb-ft	
Overturning Resistance:								
Wall Weight =	386	plf	@ y = 1.17	ft	M =	450	lb-ft	
Footing Weight =	233	plf	@ y = 1.17	ft	M =	271	lb-ft	
Soil Weight =	360	plf	@ y = 2.00	ft	M =	719	lb-ft	
DL from Above =	185	plf	@ y = 1.42	ft	M =	262	lb-ft	
				Tot	al Mr =	: 1702	lb-ft	
Bearing:					FS =	: 1.91	OK >	1.5
e =	0.77	ft						
B/6 =	0.39	ft		Foot	ing Co	nnectior	1:	
Bearing Pressure =	1950	psf			Mu =	: 17.1	k-in	
Ratio =	1.30		OK @ 4/3	Loc	ation =	C	(C or (0)
Rear Footing Bending:					u – Ru =	· 5.75 · 0.048	lii ksi	
Mu =	2.30	k-in	R	ebar G	rade =	60	ksi	
S =	128	in3			F'c =	5	ksi	
fb =	18	psi			B1 =	• 0.8		
Fb =	318	psi	ОК		m =	15.00		
Sr =	7	in3			rho =	8000.0		
Req'd Thickness =	1.9	in	ОК	rho	o min =	0.0011		
·					As =	0.07	in2/ft	
Wall Check:				Bar	Size =	4	5	
Mu (18" above TOS) =	3.90	k-in		Bar	Area =	0.20	0.31	in2
A =	36	in2	1	Minimu	m Ld =	10.2	12.7	in
S =	160	in3		Actu	al Ld =	5.0	5.0	in
fa =	12	psi	Dov	vel Spa	acing =	: 16	20	in
fb =	24	psi	Ho	ook Spa	acing =	: 32	50	in
Fb =	500	psi	OK	•	0			
ft =	13	psi						
Ft =	25	psi	ОК					
		-						